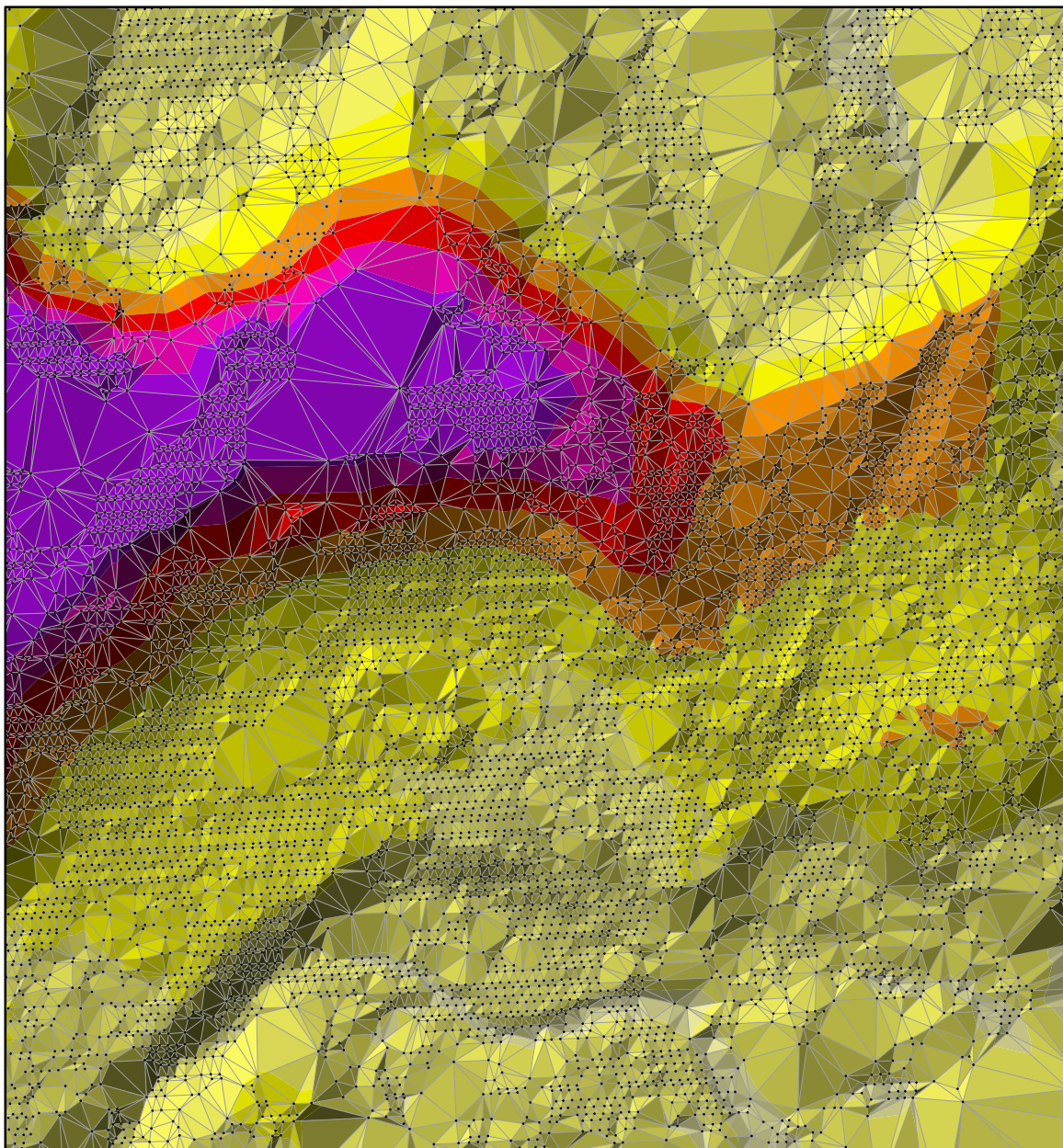


# Quality Assurance Report

## ENPLAN LIDAR SURVEY CAMPAIGN



January 2006



## EXECUTIVE SUMMARY

In the summer of 2004, Sanborn was contracted by ENPLAN to execute a LIDAR (Light Detection and Ranging) survey campaign covering portions of Shasta, Tehama, and Siskiyou counties in California. LIDAR data in the form of three-dimensional positions of a dense set of masspoints was collected for the areas given by the project specifications. The masspoint data was used in the development of digital elevation models (DEMs) delivered for first return, last return, and bare earth datasets.

The LIDAR operation was designed to provide a ground surface dataset with a vertical accuracy of 0.15m (0.5') at 1 sigma. This accuracy specification is equal to that of Sanborn's ALTM (Airborne Laser Terrain Mapping) LIDAR system manufactured by Optech and used for this project. In practice, our refined operational procedures and rigorous post-processing yield accuracies that typically exceed this specification.

The system was calibrated by conducting flight passes over a known ground surface before and after each LIDAR mission. During final data processing, the calibration parameters were inserted into post-processing software. Validation testing of the data showed that accuracy goals were exceeded with an average RMS error of 0.070m (0.23') over 28 test runs. These results were commensurate with Enplan's specified accuracy objective for the campaign.

A control network of five airborne GPS (Global Positioning System) base stations was used in this project. The network design was in accordance with the quality assurance specifications set by Sanborn's licensed surveyors, California included, as well as recommendations by Optech. Four base stations were set on National Geodetic Survey (NGS) markers located at or near the airports. Station 701 was set on the NGS monument at the East North-East side of the Benton Airport. Station 702 was set on the NGS monument at Redding Municipal Airport. Point 801 and 803 were set up on the NGS monuments at the Mott and Bidwell Airports respectively. Point 802 was also set up on an NGS monument in the project area. The above stations were tied to three additional CORS stations to create a GPS network at the common datum. The network observations and adjustments were completed on the GRS80 ellipsoid.

The acquired LIDAR data was initially processed to obtain first and last return point data. Return Intensity values were also provided within the first return point data. The last return data was then further filtered to yield a LIDAR surface representing bare earth. All three datasets were subsequently delivered to Enplan.

The contents of this report summarize the methods used to establish the base station network, perform the LIDAR data collection and post-processing, as well as the results of these methods.

Sanborn's Project Team responsible for production consisted of:

Karen Adkins - Project Manager  
Keith Kirkby - LiDAR Manager/Geodetic Engineer  
David Sutherland – Geodetic Engineering Technician  
Doug Novy – Programmer Analyst/LiDAR Processing and QC

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### APPENDIX A - FULLY CONSTRAINED LEAST SQUARES ADJUSTMENT

## 1. INTRODUCTION

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This report contains the technical description of the ENPLAN LIDAR campaign, including system calibration techniques, the establishment of base stations by a differential GPS network survey, collection and post-processing of the LIDAR data, and QA/QC results.

### 1.1 Duration and Time Period

The LIDAR aircraft arrived on site July 23, 2004 and the LIDAR data collection was accomplished between this date and July 28, 2004. The Benton, Redding, Mott, and Bidwell airports were used as the airfields of operation.

### 1.2 Contact

Questions regarding the technical aspects of this report should be addressed to:

**Sanborn**

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Colorado Springs, CO 80920

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FAX: 1-719-528-5093

Email: alucero@sanborn.com

### 1.3 Purpose of the LIDAR Acquisition

This LIDAR operation was designed to provide first return, last return, and bare earth datasets for the area of coverage specified by ENPLAN .

### 1.4 Project Location

The project location is defined by a full tile layout of the coverage area which included portions of Shasta, Tehama, and Siskiyou counties.

### 1.5 Project Scope

The ENPLAN campaign was designed to collect LIDAR derived masspoints at an approximate ground spacing of 1.5 meters or less within the project area. The data was processed to facilitate the generation of an appropriate DEM for topographic mapping and other geospatial representations.

### 1.6 Datum Issues

Five existing NGS monuments were set to be used as Airborne GPS base stations for this project. In order to obtain accurate horizontal and vertical coordinates for these stations and adjust them to a common datum, a ground control network was surveyed using GPS and tied to 3 CORS stations. A fully constrained adjustment was run on the network defining the horizontal and

vertical datums through the published coordinates and heights of NGS monuments.

#### **1.6.1 Horizontal Datum**

The horizontal datum associated with the LIDAR data is NAD83 (1992), as realized by the physical control monuments used to constrain the survey control network.

#### **1.6.2 Vertical Datum**

The vertical datum associated with the LIDAR data is NAVD88, as realized by the physical benchmarks used to constrain the survey control network.

## **2. LIDAR CALIBRATION**

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### **2.1 Introduction**

LIDAR calibrations are performed to determine and therefore eliminate systematic biases that occur within the hardware of the ALTM system. Once the biases are determined they can be modeled out. The systematic biases that are corrected for include scale, roll, and pitch.

The following procedures are intended to eliminate potential mistakes in the field and office work, and are designed to detect inconsistencies. The emphasis is not only on the quality control (QC) aspects, but also on the documentation, i.e., on the quality assurance (QA).

### **2.2 Standard Calibration Procedures**

Sanborn performs two types of calibrations on its LIDAR system. The first is a building calibration. New calibration parameters are computed and compared with previous calibration runs. If there is any change, the new values are updated internally or during the LIDAR post-processing. These values are applied to all data collected with this particular plane/ALTM system configuration.

Second, once final processing calibration parameters are established from the building data, a precisely-surveyed surface is observed with the LIDAR system to check for stability in the system. This is done several times during each mission. An average of the systematic biases is applied on a per mission basis.

#### **2.2.1 Building Calibration**

The rooftop of a large, flat, rectangular building was surveyed on the ground using conventional survey methods, and used as the LIDAR calibration target. Several specified passes were flown over the building with the ATLM system set in both scan and profile (fixed scan angle) modes.

Figure 1 shows a typical pass over the center of a building. The purpose of this pass is to identify a systematic bias in the scale of the system.

Figure 2 shows a pass along a distinct edge of a building to verify the roll compensation performed by the INS.

Additionally, a pass is made in profile mode across the middle of the building to compensate for any bias in pitch.

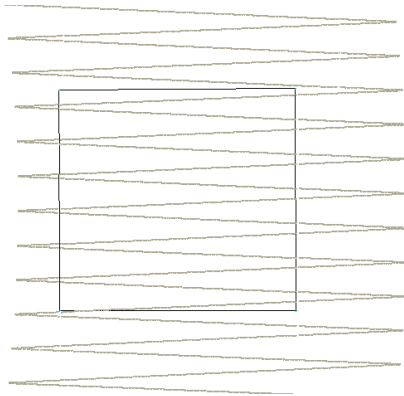


Figure 1. Calibration Pass 1

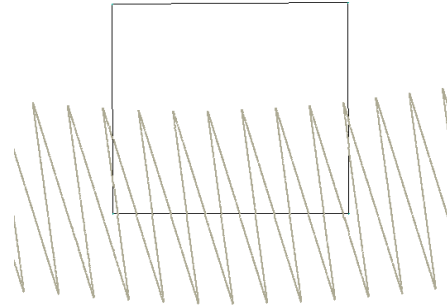


Figure 2. Calibration Pass 2

### 2.2.2 Runway Calibration and System Performance Validation

For the purpose of the ENPLAN lidar campaign, active asphalt runways were precisely-surveyed at Benton, Redding, Mott, and Bidwell airports using kinematic GPS survey techniques (accuracy:  $\pm 3\text{cm}$  at  $1\sigma$ , along each coordinate axis) to establish an accurate digital terrain model of the runway surface. The LIDAR system was then flown at right angles over the runway several times and residuals were generated from the processed data. Figure 3 shows a typical pass over the runway surface.

Approximately 25,000 LIDAR points are observed with each pass. These points are “draped” over the runway surface TIN (Triangular Irregular Network) to compute vertical residuals for every data point. The residuals are then analyzed with respect to the location *along* the runway to identify the level of noise and system biases.

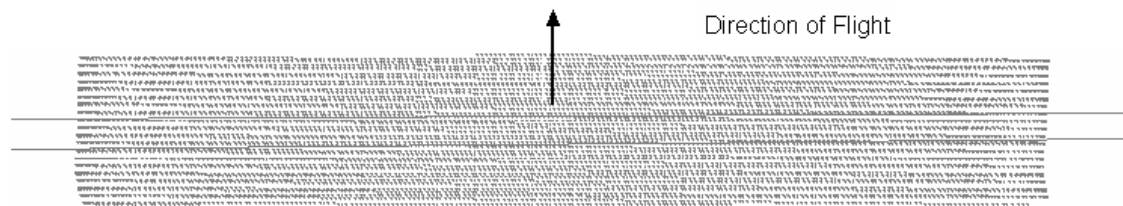


Figure 3. Runway Calibration

The runway over-flights are intended to be a quality check on the calibration and to identify any system irregularities and the overall noise. IMU misalignments and internal system calibration parameters are verified by comparing the collected LIDAR points with the runway surface.

Figure 4 shows the typical results of a runway over-flight analysis. The X-axis represents the position *along* the runway. The overall statistics from this analysis provides evidence of the overall random noise in the data (typically, 7cm standard deviation – an unbiased estimator, and 8cm RMS which includes any

biases) and indicate that the system is performing within specifications. As described in later sections of this report, this analysis will identify any peculiarities within the data along with mirror-angle scale errors (identified as a “smile” or “frown” in the data band) or roll biases.

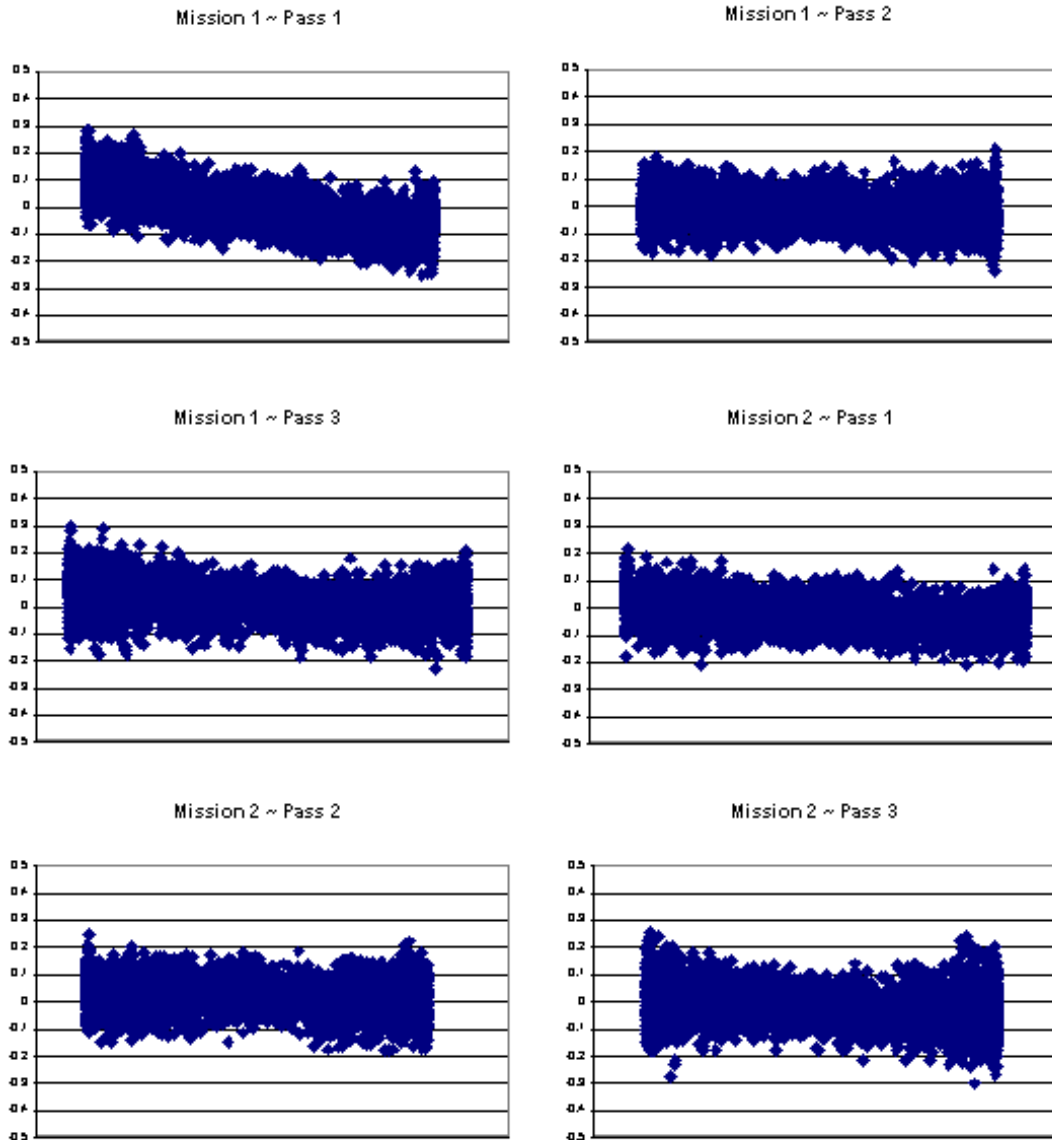


Figure 4. Runway Calibration Results



### 3. Geodetic Base Network

#### 3.1 Network Scope

Five existing NGS monuments were used as Airborne GPS base stations. The points were chosen within project boundaries, where a GPS receiver can be set up and left to log data for the duration of the LIDAR flight mission. Three additional CORS stations were involved in network adjustment.

During the LIDAR campaign, the Sanborn field crew conducted a GPS field survey to establish a survey network (Figure 5) containing the GPS base stations used to support the campaign. Point number 701 is an existing NGS monument located at Benton Airport. Point number 702 is an existing NGS monument located at Redding Airport. Also, point numbers 801 and 803 are the existing monuments located at Mott and Bidwell airports respectively. Point 802 is also a NGS monument in the project area that was used as an airborne GPS base station. See Table 2 for station names, orders and constraints.

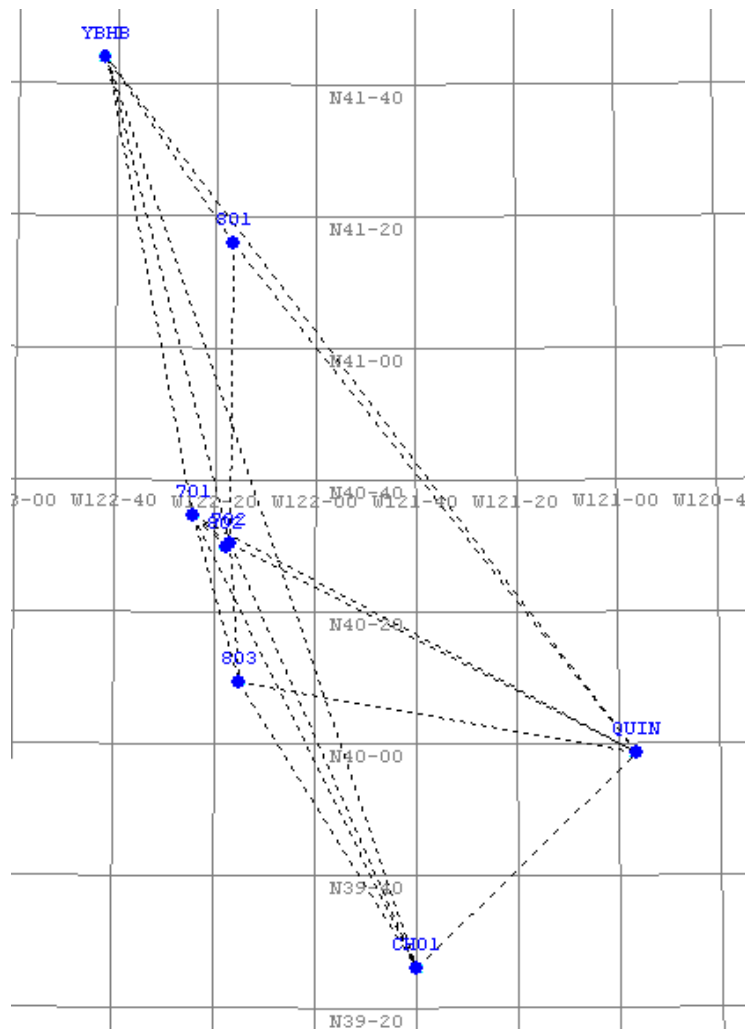


Figure 5. Survey Network Diagram



### 3.2 Data Processing and Network Adjustment

All static baseline vectors were processed using Trimble Navigation's GPSurvey™ (Ver. 2.35a) software. Fixed bias solutions were obtained for all baselines. The broadcast ephemeris was used, since the accuracy and extent of the network does not warrant the use of the precise ephemeris.

The loop misclosures are summarized in Table 1 below.

The misclosures in each component (X, Y and Z) are given in millimeters and parts per million (ppm) in an ECEF Cartesian coordinate system. The spatial misclosure in ppm is also provided. All loops comprise of quasi-independent baselines from at least two different sessions. Every station in the network appears at least once in a loop. All loops, in fact, satisfy GPS guidelines for first order work, namely:

- in any component (X, Y, Z), the maximum misclosure does not exceed 250 mm ,
- in any component (X, Y, Z), the maximum misclosure in terms of the loop length does not exceed 12.5 ppm,
- in any component (X, Y, Z), the average misclosure in terms of the loop length does not exceed 8 ppm.

**Table 1. Survey Loop Closure Summary**

<b>Loop</b>	<b>dX (cm)</b>	<b>dY (cm)</b>	<b>dZ (cm)</b>	<b>Dist. (m)</b>	<b>ppm</b>
701-YBHB-801-802-701	1.5	3.4	1.1	292097	0.40
803-701-802-801-702-803	2.4	4.9	6.6	270121	0.32
803-702-801-QUIN-803	4.6	4.8	0.6	419156	0.16
CHO1-803-QUIN-CHO1	0.9	0.2	0.1	294206	0.03

A 3-dimensional network adjustment was carried out using GeoLab™ (version 3.61) 3-D adjustment software. The network is displayed in Figure 5.

Initially, a minimally constrained adjustment was performed to examine the internal accuracy of the network. The geodetic latitude, longitude, and elevation of one existing control point were held fixed. The adjustment comprises 8 stations and 33 baseline vector components (11 baselines). *A priori* weights for the observations were based on the (scaled) variance-covariance sub-matrices from the GPSurvey™ solutions.

The relative confidence regions and the associated relative horizontal and vertical precisions were computed for all pairs of points that were directly connected by vectors. All station pairings with the exception of one (801 to 702)

meet the horizontal positioning standard for *first order* surveys, i.e., the relative horizontal precision between each pair of points does not exceed 10 mm + 10 ppm of their horizontal separation, at the 95 percent level of confidence. The network is therefore classified as *first order* in terms of its *internal* accuracy.

To complete a fully constrained adjustment, the network was horizontally constrained to control points 701, 702, QUIN, YBHB and CHO1 and vertically constrained by orthometric elevation to 901 and 801. See Table 2 for associated orders and assigned standard deviations.

TABLE 2. NETWORK ADJUSTMENT CONSTRAINTS  
(standard deviations in meters)

**Horizontal**

<b>Code</b>	<b>NGS Station Name</b>	<b>Order</b>	$\phi$	$\lambda$
701	REDDING	1	0.01	0.01
702	REDDING CBL O C	2	0.04	0.04
QUIN	QUINCY CORS L1	A(SPECIAL)	0.04	0.04
YBHB	YREKA CORS L1	A(SPECIAL)	0.04	0.04
CHO1	CHOCO 1 L1	A(SPECIAL)	0.04	0.04

**Vertical**

<b>Code</b>	<b>NGS Station Name</b>	<b>Order</b>	<b>Ht</b>
801	MOTT AIRPORT	1 – II	0.01
802	P 742	1 – II	0.05
803	T 742	1 – II	0.05
701	FAA GAF B	1 – II	0.05

A full listing of the constrained adjustment is contained in Appendix A. The residuals and the standardized residuals are also listed in Appendix A. One of the 33 vector components were flagged for possible rejection under the  $\tau$ MAX - test at the 0.05 level of significance. None of the horizontal or vertical constraints were flagged. The slight change in the a posteriori variance factor ( $\sigma_o^2 = 1.0434$ ) from the minimally constrained adjustment indicates that the network is not being unduly distorted by the imposition of the constraints. The absolute and relative confidence regions were not scaled by the a posteriori variance factor. The relative horizontal confidence ellipses appear in last sections of Appendix A. Examination of the relative precision reveals that the network has maintained its high internal accuracy.

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## 4. LIDAR DATA CAPTURE

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### 4.1 Field Work and Procedures

Data capture began July 23 and was completed July 28, 2004. A minimum of two GPS base stations were set up and operated continuously during capture. One receiver was usually set up at the airport of operation while the second GPS receiver was placed at a survey control point in the area of capture.

Pre-flight checks such as cleaning the sensor head glass were performed. A four minute INS initialization is conducted on the ground, with the engines running, prior to flight, to establish fine-alignment of the INS. GPS ambiguities are resolved by flying within ten kilometers of the base stations.

The flight missions were typically four or five hours in duration including runway calibration flights flown at the beginning and the end of each mission. During the data collection, the operator recorded information on logsheets which includes weather conditions, LIDAR operation parameters, and flight line statistics. Near the end of the mission GPS ambiguities are again resolved by flying within ten kilometers of the base stations to aid in post-processing.

Table 3 shows the planned LIDAR acquisition parameters with a flying height of 1,200 meters above ground level (agl) on a mission to mission basis.

**Table 3. LIDAR Acquisition Parameters**

Average Altitude	1,200 Meters Above Ground Level
Airspeed	~ 140 Knots
Scan Frequency	33 Hertz
Scan Width Half Angle	20 Degrees
Pulse Rate	50000 Hertz

Preliminary data processing was performed in the field immediately following the missions for quality control of GPS data and to ensure sufficient overlap between flight lines. Any problematic data could then be reflown immediately as required. Final data processing was completed in the Colorado Springs office.

### 4.2 Final LIDAR Processing

Final post-processing of LIDAR data involves several steps. The airborne GPS data was post-processed using Waypoint's GravNAV™ software (version 7.50). A fixed-bias carrier phase solution was computed in both the forward and reverse chronological directions. Whenever practical, LIDAR acquisition was limited to periods when the PDOP was less than 4.0.

The GPS trajectory was combined with the raw IMU data and post-processed using Applanix POSPROC Kalman Filtering software. This generally results in a two-fold improvement in the attitude accuracies over the real-time INS data. The

best estimated trajectory (BET) and refined attitude data were then re-introduced into the Optech REALM software to compute the laser point-positions. The trajectory was combined with the attitude data and laser range measurements to produce the 3-dimensional coordinates of the mass points.

First and last return values were produced within REALM software. The first return information provides a useful depiction of the “canopy” within the project area. The last return was further processed to obtain ground-filtered data with a corresponding regular grid DEM.

Laser point filtering was accomplished using TerraScan LIDAR processing and modeling software. The bare earth surface generated by TerraScan was used to produce regular grid DEMs.

#### **4.3 Problems and Delays**

No major problems or challenges occurred for the duration of the project.

#### **4.4 Daily Runway Performance and Data Validation Tests**

Performance flights over the runway test field were performed before and after each mission. Table 4 shows the standard deviation and RMS values of the residuals between the test flights and the known surface of the test ranges for each pass. The maximum RMS value is 0.104 meters and the maximum standard deviation is 0.059 meters. The average RMS among all test flights is 0.070 meters. Figure 4, above, provides a graphical representation of the runway results.

Rigorous quality assurance procedures were followed to ensure that the appropriate data accuracy was achieved.

Table 4. Runway Validation Results (meters)

Mission	Pass	Standard Deviation	RMS
204a	1	0.057	0.060
204a	2	0.053	0.054
205a	1	0.046	0.104
205a	2	0.046	0.083
205a	3	0.056	0.103
205a	4	0.050	0.095
205b	1	0.059	0.097
205b	2	0.058	0.101
205b	3	0.055	0.096
205b	4	0.051	0.094
207a	1	0.049	0.071
207a	2	0.049	0.065
207a	3	0.048	0.049
207b	1	0.046	0.049
207b	2	0.044	0.051
207b	3	0.051	0.052
207b	4	0.048	0.063
208a	1	0.043	0.074
208a	2	0.052	0.067
208a	3	0.042	0.075
208a	4	0.043	0.065
208b	1	0.043	0.054
208b	2	0.046	0.056
208b	3	0.046	0.054
208b	4	0.048	0.055
209a	1	0.050	0.074
209a	2	0.047	0.047
209a	3	0.045	0.061

## **APPENDIX A**

### **FULLY CONSTRAINED LEAST SQUARES ADJUSTMENT**

Appendix A FC adjustment.txt

```

=====
Redding, CA - Full Constrained NAD83(1992)
GeoLab V3.65      WGS 84      UNITS: m, DMS      Page 0001
=====
14:11:18, Tue Oct 05, 2004

```

```

Input file: J:\Redding\GeoLab\Redding_C.iob
Output file: J:\Redding\GeoLab\Redding_C.lst
Options file: C:\glab32v3\default.cfg

```

Geoid File: C:\GEOID\G2003U05PC.gsp

PARAMETERS		OBSERVATIONS	
Description	Number	Description	Number
No. of Stations	8	Directions	0
Coord Parameters	24	Distances	0
Free Latitudes	8	Azimuths	0
Free Longitudes	8	Vertical Angles	0
Free Heights	8	Zenithal Angles	0
Fixed Coordinates	0	Angles	0
Astro. Latitudes	0	Heights	4
Astro. Longitudes	0	Height Differences	0
Geoid Records	0	Auxiliary Params.	0
All Aux. Pars.	0	2-D Coords.	10
Direction Pars.	0	2-D Coord. Diffs.	0
Scale Parameters	0	3-D Coords.	0
Constant Pars.	0	3-D Coord. Diffs.	33
Rotation Pars.	0		
Translation Pars.	0		
	-----		-----
Total Parameters	24	Total Observations	47
Degrees of Freedom = 23			

SUMMARY OF SELECTED OPTIONS

OPTION	SELECTION
Computation Mode	Adjustment
Maximum Iterations	5
Convergence Criterion	0.00100
Angular Misclosure Limit Factor	0.00
Linear Misclosure Limit Factor	0.00
Residual Rejection Criterion	Tau Max
Confidence Region Types	1D 2D 3D Station Relative
Relative Confidence Regions	Connected Only
Variance Factor (VF) Known	Yes
Scale Covariance Matrix With VF	No
Scale Residual Variances With VF	No
Force Convergence in Max Iters	No
Distances Contribute To Heights	No
Compute Full Inverse	Yes
Optimize Band Width	Yes
Generate Initial Coordinates	Yes
Re-Transform Obs After 1st Pass	Yes
Geoid Interpolation Method	Bi-Quadratic



Appendix A FC adjustment.txt

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GeoLab V3.65	Redding, CA - Min. Constrained NAD83(1992)	UNIT S: m, DMS	Page 0002
	WGS 84		

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Input Station Data:

FFF STATION	ELIP-LATITUDE ASTRO-LATITUDE N/S DEFLECTION NORTHING	ELIP-LONGITUDE ASTRO-LONGITUDE N/S DEFLECTION EASTING	ELIP-HEIGHT ORTHO-HEIGHT GEOID-HEIGHT PROJECTION
000 701	N 40 34 36.02678 N 40 34 36.02678 - 0 0 8.37 4491935.810	W122 24 17.24782 W122 24 17.24782 0 0 0.93 550377.025	193.465 218.280 -27.407 UTM 10
000 702	N 40 30 16.00784 N 40 30 16.00784 - 0 0 6.35 4483993.195	W122 17 9.30696 W122 17 9.30696 - 0 0 3.56 560503.124	121.290 121.290 -27.643 UTM 10
000 801	N 41 15 59.67819 N 41 15 59.67819 - 0 0 7.34 4568604.041	W122 16 28.62730 W122 16 28.62730 0 0 2.21 560759.437	968.457 993.183 -23.760 UTM 10
000 802	N 40 29 54.42885 N 40 29 54.42885 - 0 0 6.68 4483321.720	W122 17 41.61083 W122 17 41.61083 - 0 0 3.54 559748.141	124.269 149.340 -27.678 UTM 10
000 803	N 40 9 20.03529 N 40 9 20.03529 - 0 0 2.11 4445292.587	W122 15 3.49135 W122 15 3.49135 - 0 0 8.70 563791.973	78.715 104.700 -28.509 UTM 10
000 CH01	N 39 25 57.46627 N 39 25 57.46627 - 0 0 7.06 4365638.061	W121 39 53.86634 W121 39 53.86634 - 0 0 2.75 614897.943	17.750 17.750 -28.187 UTM 10
000 QUI N	N 39 58 28.37874 N 39 58 28.37874 - 0 0 1.08 4426955.919	W120 56 39.88710 W120 56 39.88710 - 0 0 5.05 675536.059	1108.005 1108.005 -23.838 UTM 10
000 YBHB	N 41 43 53.97171 N 41 43 53.97171 0 0 1.21 4620023.731	W122 42 38.64701 W122 42 38.64701 - 0 0 0.10 524056.612	1068.057 1068.057 -23.635 UTM 10

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GeoLab V3.65	Redding, CA - Min. Constrained NAD83(1992)	UNIT S: m, DMS	Page 0003
	WGS 84		

=====

Misclosures (pass 1):

NOTE: Observation values shown are reduced to mark-to-mark.

TYPE	AT	FROM	TO	OBSERVATION	STD. DEV.	MISC
ELAT	701			N 40 34 36.02678	0.010	0.000
ELON	701			W122 24 17.24782	0.010	0.000
ELAT	702			N 40 30 15.95324	0.040	1.682
ELON	702			W122 17 9.22334	0.040	-1.971

# Apendi x A FC adjustment. txt

ELAT	QUIN	N 39 58	28. 37579	0. 040	0. 090
ELON	QUIN	W120 56	39. 88402	0. 040	-0. 075
ELAT	YBHB	N 41 43	53. 94101	0. 040	0. 952
ELON	YBHB	W122 42	38. 59169	0. 040	-1. 275
ELAT	CH01	N 39 25	57. 48084	0. 040	-0. 461
ELON	CH01	W121 39	53. 80602	0. 040	-1. 439
OHGT	801		993. 183	0. 001	-0. 967
OHGT	802		149. 340	0. 050	2. 607
OHGT	803		104. 700	0. 050	2. 524
OHGT	701		218. 280	0. 050	2. 592
GROUP:	00000041. SSF, obs#:	1 day 210	OPT	210	22
DXCT	701	802	4873. 299	0. 013	-0. 230
DYCT	701	802	-9707. 191	0. 017	0. 024
DZCT	701	802	-6646. 297	0. 011	-0. 120
GROUP:	00000021. SSF, obs#:	2 day 211	OPT	211	1
DXCT	701	803	-5108. 779	0. 012	-0. 230
DYCT	701	803	-32504. 620	0. 016	0. 024
DZCT	701	803	-35702. 799	0. 016	-0. 120
GROUP:	00000025. SSF, obs#:	3 day 210	OPT	210	20
DXCT	702	802	-874. 353	0. 002	-0. 811
DYCT	702	802	43. 440	0. 002	-4. 460
DZCT	702	802	-505. 392	0. 002	1. 188
GROUP:	00000228. SSF, obs#:	4 day 206	OPT	206	
DXCT	702	803	-10856. 437	0. 015	-0. 806
DYCT	702	803	-22754. 004	0. 023	-4. 445
DZCT	702	803	-29561. 913	0. 020	1. 207
GROUP:	00000236. SSF, obs#:	5 day 210	OPT	210	1
DXCT	801	702	-30051. 654	0. 009	0. 018
DYCT	801	702	-45791. 381	0. 011	1. 488
DZCT	801	702	-64546. 605	0. 011	0. 754
GROUP:	00000220. SSF, obs#:	6 day 204	OPT	204	1
DXCT	801	QUIN	46847. 647	0. 022	-0. 219
DYCT	801	QUIN	-138643. 169	0. 028	-2. 323
DZCT	801	QUIN	-108829. 215	0. 028	1. 507
GROUP:	00000224. SSF, obs#:	7 day 204	OPT	204	1
DXCT	801	YBHB	-12369. 108	0. 015	-1. 005
DYCT	801	YBHB	48375. 355	0. 021	-1. 058
DZCT	801	YBHB	38756. 693	0. 022	2. 182
GROUP:	00000256. SSF, obs#:	8 day 207	OPT	207	2
DXCT	803	QUIN	87755. 692	0. 012	0. 615
DYCT	803	QUIN	-70097. 834	0. 015	0. 684
DZCT	803	QUIN	-14720. 691	0. 014	-0. 459
GROUP:	00000232. SSF, obs#:	9 day 207	OPT	207	2
DXCT	CH01	803	-15416. 799	0. 016	0. 306
DYCT	CH01	803	70114. 805	0. 019	-1. 782
DZCT	CH01	803	61711. 457	0. 018	1. 533
GROUP:	00000244. SSF, obs#:	10 day 208	OPT	208	
DXCT	CH01	QUIN	72338. 884	0. 011	0. 929
DYCT	CH01	QUIN	16. 973	0. 015	-1. 100
DZCT	CH01	QUIN	46990. 767	0. 015	1. 073
GROUP:	00000240. SSF, obs#:	11 day 210	OPT	210	22
DXCT	YBHB	701	-23430. 165	0. 020	0. 408
DYCT	YBHB	701	-84416. 104	0. 028	-1. 939
DZCT	YBHB	701	-97162. 365	0. 027	-0. 148

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Adjusted NEO Coordinates:

CODE	FF	STATION	NORTHING STD DEV	EASTING STD DEV	O-HEIGHT STD DEV	MAPPROJ
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Apendi x A FC adjustment. txt

NEO 000 701	4491935. 815	550377. 023	218. 333	UTM 10	m
	0. 009	0. 009	0. 016		
NEO 000 702	4483991. 543	560505. 041	150. 367	UTM 10	m
	0. 012	0. 010	0. 012		
NEO 000 801	4568603. 757	560760. 554	993. 183	UTM 10	m
	0. 012	0. 011	0. 001		
NEO 000 802	4483321. 880	559748. 347	149. 412	UTM 10	m
	0. 012	0. 010	0. 012		
NEO 000 803	4445292. 715	563792. 186	104. 680	UTM 10	m
	0. 012	0. 010	0. 018		
NEO 000 CH01	4365638. 501	614899. 371	45. 408	UTM 10	m
	0. 013	0. 012	0. 025		
NEO 000 QUI N	4426955. 812	675536. 145	1130. 283	UTM 10	m
	0. 013	0. 012	0. 021		
NEO 000 YBHB	4620022. 762	524058. 005	1090. 119	UTM 10	m
	0. 013	0. 012	0. 025		

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Adjusted PLH Coordinates:

CODE	FFF	STATION		LATITUDE STD DEV		LONGITUDE STD DEV		ELIP-HEIGHT STD DEV	
PLH	000	701	N 40 34	36. 02691	W122 24	17. 24788		190. 926 m	0
				0. 009		0. 009		0. 016	
PLH	000	702	N 40 30	15. 95376	W122 17	9. 22607		122. 724 m	0
				0. 012		0. 010		0. 012	
PLH	000	801	N 41 15	59. 66868	W122 16	28. 57939		969. 423 m	0
				0. 012		0. 011		0. 001	
PLH	000	802	N 40 29	54. 43400	W122 17	41. 60200		121. 734 m	0
				0. 012		0. 010		0. 012	
PLH	000	803	N 40 9	20. 03938	W122 15	3. 48232		76. 171 m	0
				0. 012		0. 010		0. 018	
PLH	000	CH01	N 39 25	57. 47985	W121 39	53. 80633		17. 220 m	0
				0. 013		0. 012		0. 025	
PLH	000	QUI N	N 39 58	28. 37520	W120 56	39. 88359		1106. 445 m	0
				0. 013		0. 012		0. 021	
PLH	000	YBHB	N 41 43	53. 94017	W122 42	38. 58686		1066. 485 m	0
				0. 013		0. 012		0. 025	

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Geoid Values:

CODE	STATION	N/S DEFLECTION	E/W DEFLECTION	UNDULATION
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Apendi x A FC adjustment. txt

GEOI	701	-	0	0	8.37	0	0	0.93	-27.407 m
GEOI	702	-	0	0	6.35	-	0	0	-27.643 m
GEOI	801	-	0	0	7.34	0	0	2.21	-23.760 m
GEOI	802	-	0	0	6.68	-	0	0	-27.678 m
GEOI	803	-	0	0	2.11	-	0	0	-28.509 m
GEOI	CH01	-	0	0	7.06	-	0	0	-28.187 m
GEOI	QUI N	-	0	0	1.08	-	0	0	-23.838 m
GEOI	YBHB		0	0	1.21	-	0	0	-23.635 m

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WGS 84      UNIT S: m, DMS

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Residuals (critical value = 3.135):

NOTE: Observation values shown are reduced to mark-to-mark.

TYPE	AT	FROM	TO	OBSERVATION STD DEV	RESIDUAL STD DEV	STD RES PPM
ELAT	701			N 40 34 36.02678 0.010	0.004 0.000	0.004
ELON	701			W122 24 17.24782 0.010	-0.001 0.000	-0.001
ELAT	702			N 40 30 15.95324 0.040	0.014 0.038	0.373
ELON	702			W122 17 9.22334 0.040	-0.062 0.039	-1.599
ELAT	QUI N			N 39 58 28.37579 0.040	-0.002 0.028	-0.073
ELON	QUI N			W120 56 39.88402 0.040	-0.020 0.038	-0.524
ELAT	YBHB			N 41 43 53.94101 0.040	-0.058 0.027	-2.172
ELON	YBHB			W122 42 38.59169 0.040	0.118 0.038	3.083
ELAT	CH01			N 39 25 57.48084 0.040	-0.020 0.027	-0.748
ELON	CH01			W121 39 53.80602 0.040	-0.013 0.038	-0.329
OHGT	801			993.18300 0.001	0.000 0.000	0.000
OHGT	802			149.34000 0.050	0.072 0.048	1.490
OHGT	803			104.70000 0.050	-0.020 0.047	-0.440
OHGT	701			218.28000 0.050	0.053 0.047	1.110
GROUP:	00000041. SSF, obs#:	1 day	210 OPT		210 22	
DXCT	701	802		4873.29940 0.013	-0.003 0.011	-0.277 0.23
DYCT	701	802		-9707.19070 0.017	-0.002 0.013	-0.157 0.17
DZCT	701	802		-6646.29690 0.011	0.003 0.007	0.518 0.27
GROUP:	00000021. SSF, obs#:	2 day	211 OPT		211 1	
DXCT	701	803		-5108.77930 0.012	-0.002 0.008	-0.198 0.03
DYCT	701	803		-32504.62000 0.016	-0.008 0.010	-0.788 0.17
DZCT	701	803		-35702.79930	-0.016	-1.363

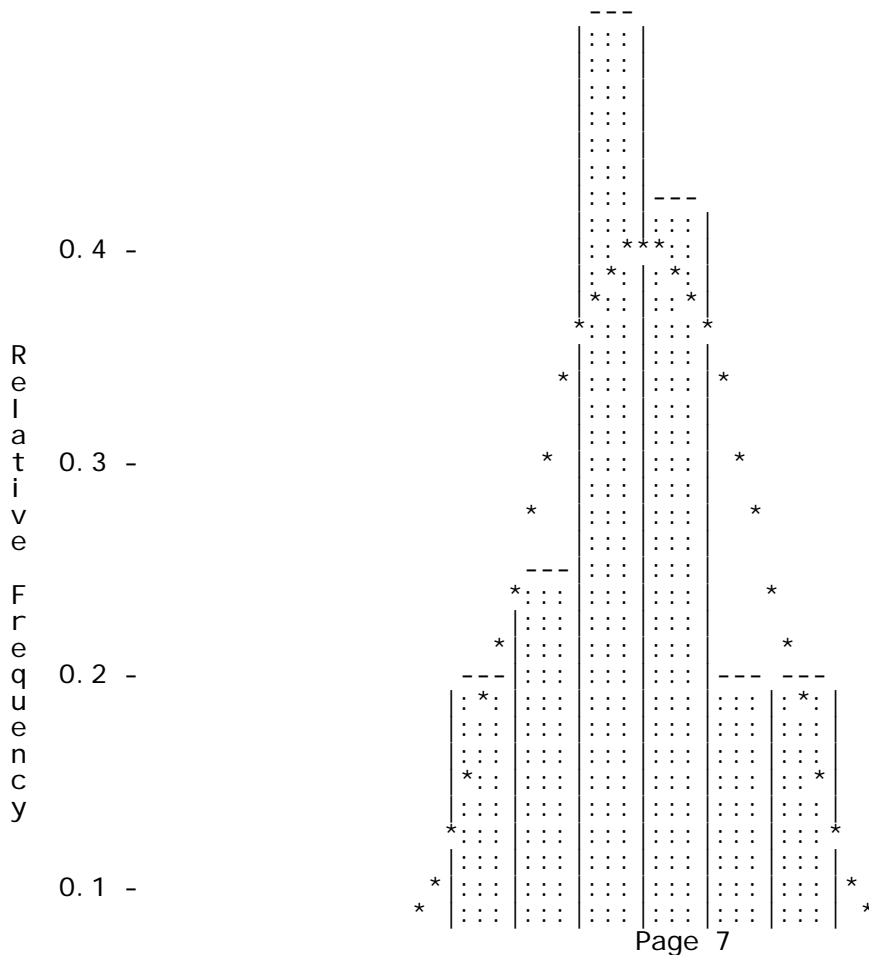


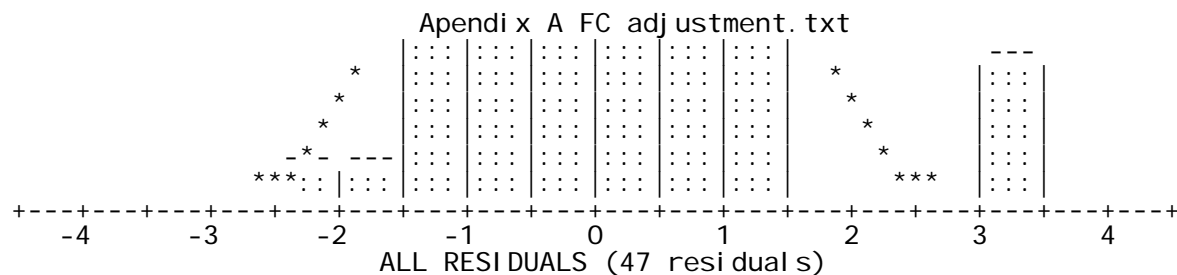
		Apendix A FC adjustment.txt				
DYCT	CH01	QUIN	16.97270	0.001	0.092	
			0.015	0.008	0.01	
DZCT	CH01	QUIN	46990.76720	0.000	0.001	
			0.015	0.008	0.00	
GROUP: 00000240. SSF, obs#:		11 day 210 OPT		210 22		
DXCT	YBHB	701	-23430.16490	-0.012	-0.793	
			0.020	0.015	0.09	
DYCT	YBHB	701	-84416.10430	0.002	0.097	
			0.028	0.021	0.02	
DZCT	YBHB	701	-97162.36550	-0.023	-1.188	
			0.027	0.020	0.18	

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### S T A T I S T I C S      S U M M A R Y

Residual Critical Value Type	Tau Max
Residual Critical Value	3.1350
Number of Flagged Residuals	1
Convergence Criterion	0.0010
Final Iteration Counter Value	2
Confidence Level Used	95.0000
Estimated Variance Factor	1.0434
Number of Degrees of Freedom	23

Chi-Square Test on the Variance Factor:

9.3233e-01 < 1.0000 < 3.0371e+00 ?

THE TEST PASSES

NOTE: All confidence regions were computed using the following factors:

Variance factor used	=	1.0000
1-D expansion factor	=	1.9600
2-D expansion factor	=	2.4477
3-D expansion factor	=	2.7955

Note that, for relative confidence regions, precisions are computed from the ratio of the major semi-axis and the spatial distance between the two stations.



Appendix A FC adjustment.txt

Redding, CA - Min. Constrained NAD83(1992)						
GeoLab V3.65	WGS 84		UNITS: m, DMS			Page 0011
2-D and 1-D Station Confidence Regions (95.000 and 95.000 percent):						
STATION	MAJOR SEMI -AXIS	AZ	MINOR SEMI -AXIS			VERTICAL
701	0.022	3	0.022			0.031
702	0.029	5	0.023			0.024
801	0.030	3	0.027			0.002
802	0.029	6	0.023			0.024
803	0.029	1	0.024			0.036
CH01	0.032	4	0.029			0.049
QUIN	0.032	3	0.029			0.042
YBHB	0.031	0	0.029			0.050

Redding, CA - Min. Constrained NAD83(1992)						
GeoLab V3.65	WGS 84		UNITS: m, DMS		Page 0012	
3D Station Confidence Regions (95.000 percent):						
STATION	MAJ-SEMI (AZ, VANG)	MED-SEMI (AZ, VANG)	MIN-SEMI (AZ, VANG)			
701	0.044 (180, 88)	0.025 ( 0, 2)	0.025 ( 90, 0)			
702	0.034 (185, 56)	0.032 ( 6, 34)	0.027 (275, 1)			
801	0.035 ( 3, 0)	0.031 ( 93, 0)	0.003 ( 0, 90)			
802	0.035 (185, 60)	0.032 ( 6, 30)	0.027 (276, 0)			
803	0.051 (223, 88)	0.033 (360, 1)	0.028 ( 90, 1)			
CH01	0.070 (258, 88)	0.037 ( 3, 1)	0.033 ( 93, 2)			
QUIN	0.060 (255, 88)	0.037 ( 2, 1)	0.033 ( 92, 2)			
YBHB	0.071 (216, 89)	0.035 ( 0, 1)	0.033 ( 90, 1)			

Redding, CA - Min. Constrained NAD83(1992)							
GeoLab V3.65		WGS 84		UNITS: m, DMS		Page 0013	
2-D and 1-D Relative Station Confidence Regions (95.000 and 95.000 percent):							
FROM	TO	MAJ-SEMI	AZ	MIN-SEMI	VERTICAL	DISTANCE	PPM
701	702	0.022	5	0.010	0.026	12877.150	1.70
701	802	0.022	5	0.009	0.026	12733.885	1.74
701	803	0.022	1	0.012	0.031	48552.461	0.45
701	CH01	0.027	3	0.021	0.047	141868.05	0.19
701	QUIN	0.027	3	0.021	0.040	141068.44	0.19
701	YBHB	0.025	1	0.020	0.052	130826.53	0.19
702	801	0.017	4	0.016	0.024	84653.501	0.21
702	802	0.005	12	0.003	0.006	1010.842	4.94
702	803	0.014	8	0.010	0.034	38852.453	0.37
702	CH01	0.023	16	0.020	0.048	130292.90	0.17
702	QUIN	0.022	12	0.020	0.041	128436.39	0.17

		Apendi x A FC adjustment. txt					
702	YBHB	0.023	179	0.020	0.052	140895.76	0.16
801	QUI N	0.025	7	0.023	0.042	182374.43	0.14
801	YBHB	0.018	170	0.015	0.050	63207.998	0.28
803	CH01	0.019	49	0.018	0.037	94668.196	0.20
803	QUI N	0.019	28	0.018	0.029	113276.06	0.16
CH01	QUI N	0.018	33	0.017	0.034	86261.505	0.20
CH01	YBHB	0.029	1	0.027	0.067	270212.06	0.11
QUI N	YBHB	0.028	178	0.026	0.062	245487.60	0.12

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WGS 84      UNITS: m, DMS

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3D Relative Confidence Regions (95.000 percent):

FROM	TO	MAJ-SEMI (AZ, VANG)	MED-SEMI (AZ, VANG)	MIN-SEMI (AZ, VANG)	PPM
		DI STANCE			
701	702	0.039 (183, 70)	0.022 ( 6, 20)	0.011 (275, 1)	
			12877.150		3.00
701	802	0.038 (183, 68)	0.022 ( 6, 22)	0.011 (276, 1)	
			12733.885		3.01
701	803	0.045 (191, 81)	0.024 ( 0, 9)	0.013 ( 90, 2)	
			48552.461		0.93
701	CH01	0.067 (212, 86)	0.031 ( 2, 4)	0.024 ( 92, 2)	
			141868.049		0.47
701	QUI N	0.058 (201, 85)	0.030 ( 1, 5)	0.024 ( 92, 2)	
			141068.445		0.41
701	YBHB	0.074 (190, 87)	0.028 ( 0, 3)	0.023 ( 90, 0)	
			130826.531		0.57
702	801	0.034 (203, 87)	0.020 ( 0, 3)	0.019 ( 90, 1)	
			84653.501		0.40
702	802	0.008 ( 0, 90)	0.006 ( 0, 0)	0.004 ( 90, 0)	
			1010.842		8.18
702	803	0.048 (188, 85)	0.016 ( 8, 5)	0.011 (278, 0)	
			38852.453		1.24
702	CH01	0.069 (217, 87)	0.026 ( 14, 3)	0.023 (104, 1)	
			130292.896		0.53
702	QUI N	0.059 (204, 87)	0.025 ( 10, 3)	0.023 (101, 1)	
			128436.392		0.46
702	YBHB	0.074 (199, 88)	0.026 ( 0, 2)	0.022 ( 90, 1)	
			140895.762		0.53
801	QUI N	0.060 (226, 88)	0.028 (360, 2)	0.027 ( 90, 2)	
			182374.431		0.33
801	YBHB	0.071 (234, 89)	0.020 (350, 1)	0.017 ( 80, 1)	
			63207.998		1.13
803	CH01	0.053 (246, 87)	0.021 ( 46, 3)	0.021 (136, 1)	
			94668.196		0.56
803	QUI N	0.042 (227, 88)	0.021 (360, 1)	0.021 ( 90, 1)	
			113276.061		0.37
CH01	QUI N	0.048 (270, 89)	0.020 ( 0, 0)	0.020 ( 90, 1)	
			86261.505		0.56
CH01	YBHB	0.095 (227, 88)	0.033 (360, 1)	0.031 ( 90, 1)	
			270212.062		0.35
QUI N	YBHB	0.088 (220, 88)	0.032 ( 0, 1)	0.030 ( 90, 1)	
			245487.595		0.36

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